

การออกแบบระบบกวนเร็วแบบ *Parshall Flume*

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1 Parshall Flume Design

Item	Unit	Apply Unit	Unit
1.1 Power (P)	KW	$W = N.m/s$	$Kg.m^2/s^3$
		$N = kg.m/s^2$	
1.2 Mass Density of water	kg/m^3	1000	kg/m^3
1.3 Flow rate (Q)	m^3/min		
1.4 Acceleation of gravity (g)	m/s^2	9.81	m/s^2
1.5 ΔH	m.		

1 Hydraulic Type

Theory

$$P = \rho g Q \Delta H$$

$$P = 1,000 \frac{\text{kg}}{\text{m}^3} 9.81 \frac{\text{m}}{\text{s}^2} \times Q (\text{m}^3 / \text{min}) \times \Delta H (\text{m})$$

$$P = 1,000 \text{kg} \times 9.81 \frac{\text{m}}{\text{s}^2} \times Q \left(\frac{1}{\text{min}} \times \frac{\text{min}}{60 \text{sec}} \right) \times \Delta H (\text{m})$$

$$P = 1,000 \times 9.81 \times \frac{1}{60} \times Q \times \Delta H \left(\text{kg} \cdot \frac{\text{m}^2}{\text{s}^3} \right)$$

$$P = 1,000 \times \left(9.81 \times \frac{1}{60} \times Q \Delta H \right) \left[\text{kg} \frac{\text{m}^2}{\text{s}^3} \right]$$

$$P = 1,000 \times (0.1635 \times Q \Delta H) \left[\text{kg} \frac{\text{m}^2}{\text{s}^3} \right]$$

$$P = 1,000 (0.1635 \times Q \Delta H) \left[\text{kg} \frac{\text{m}}{\text{s}^2} \frac{\text{m}}{\text{s}} \right]$$

$$P = 1,000 (0.1635 \times Q \Delta H) \left[\text{N} \frac{\text{m}}{\text{s}} \right]$$

$$\therefore P = 0.1635 Q \Delta H \quad [\text{KW}]$$

CHECK THE REC-TAIL CONDITION.

W, throat width	H_b/H_a ratio
3-9 in. (76-229 mm)	< 0.6
1-8 ft. (0.30-2.44 m)	< 0.7
10-50 ft. (3.05-15.2 m)	< 0.8

Under unrestricted flow conditions, the discharge through a Parshall flume is can be determined from Eq. (8.31) by using the reading of flow depth H_a .⁴⁴

$$Q = 4WH_a^{1.522}W^{0.026} \quad (8.31)$$

where

Q = free-flow discharge, cfs

W = throat width, ft.

H_a = depth of water at upstream gauging point, ft.

TABLE 8.10 Free discharge as a function of throat width

Throat width, ft	Free discharge equation, ft^3/s
0.25	$Q = 0.992 H_a^{1.547}$
0.50	$Q = 2.06 H_a^{1.58}$
0.67	$Q = 3.07 H_a^{1.53}$
$1 \leq W \leq 8$	$Q = 4 W H_a^{1.522}W^{0.026}$
$10 \leq W \leq 50$	$Q = (3.6875 W + 2.5)H_a^{1.6}$

desirable to design the Parshall flume so that free flow occurs, under some flow conditions the hydraulic jump at the exit section will be submerged, and the free-flow condition will not exist. Nonfree discharge or submerged flow occurs when

$$\frac{H_b}{H_a} \geq 0.6 \quad \text{for } W = 0.25, 0.50, 0.75 \text{ ft (0.076, 0.15, 0.23 m)}$$

$$\frac{H_b}{H_a} \geq 0.7 \quad \text{for } 1 \leq W \leq 8 \text{ ft (0.30} \leq W \leq 2.4 \text{ m)}$$

and $\frac{H_b}{H_a} \geq 0.8 \quad \text{for } 10 \leq W \leq 50 \text{ ft (0.24} \leq W \leq 15 \text{ m)}$

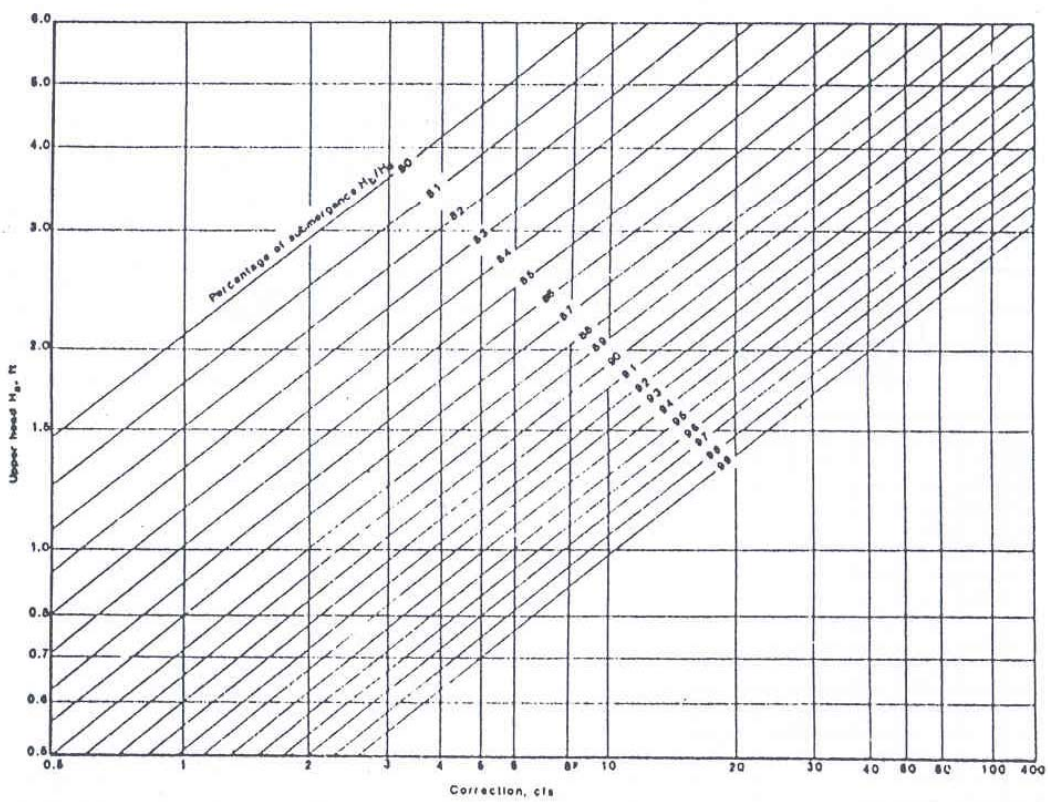
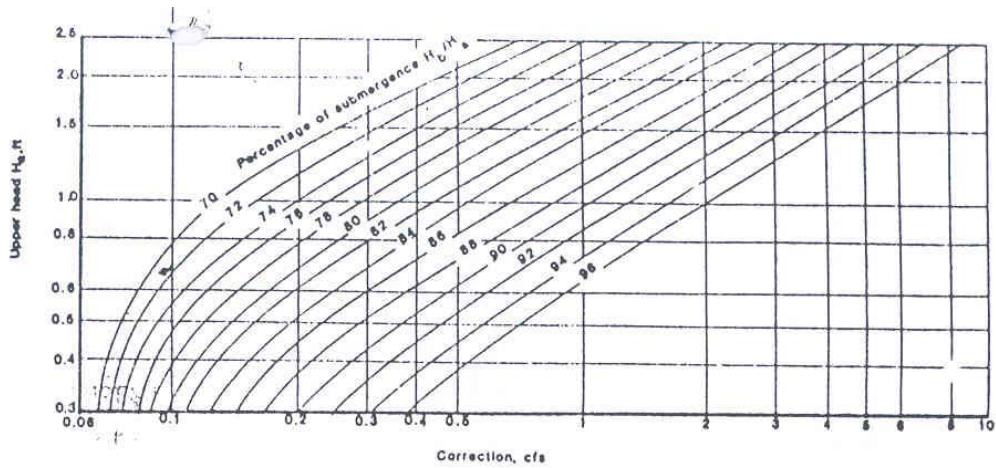


FIGURE 8.21 Diagrams for computing submerged flow through Parshall flumes of various sizes.

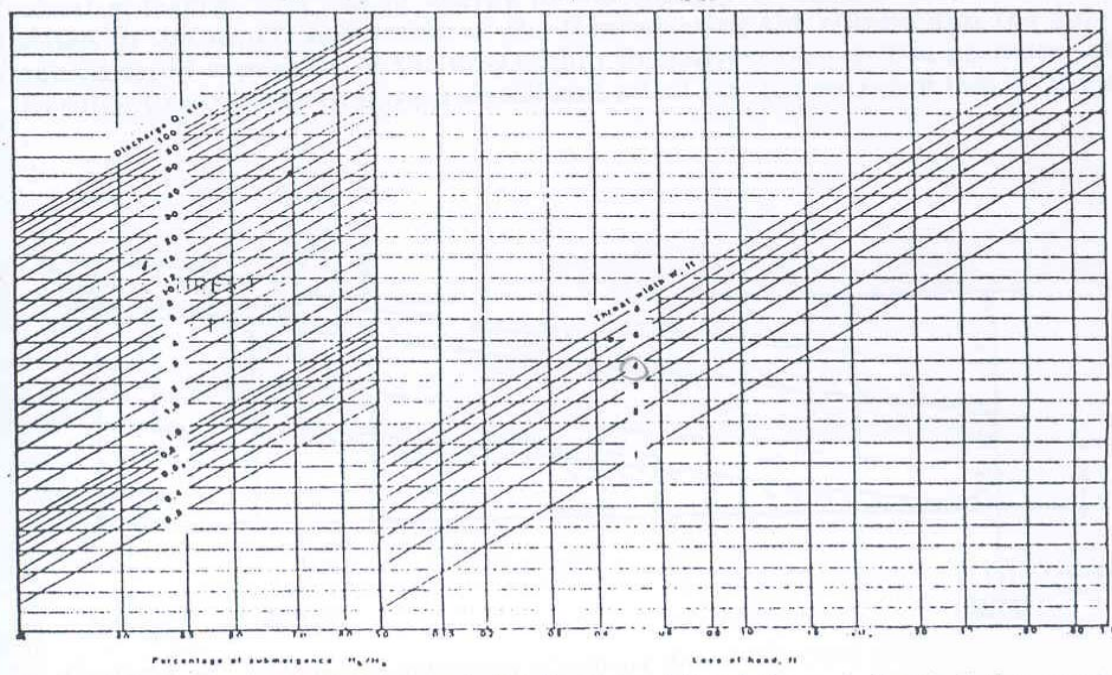
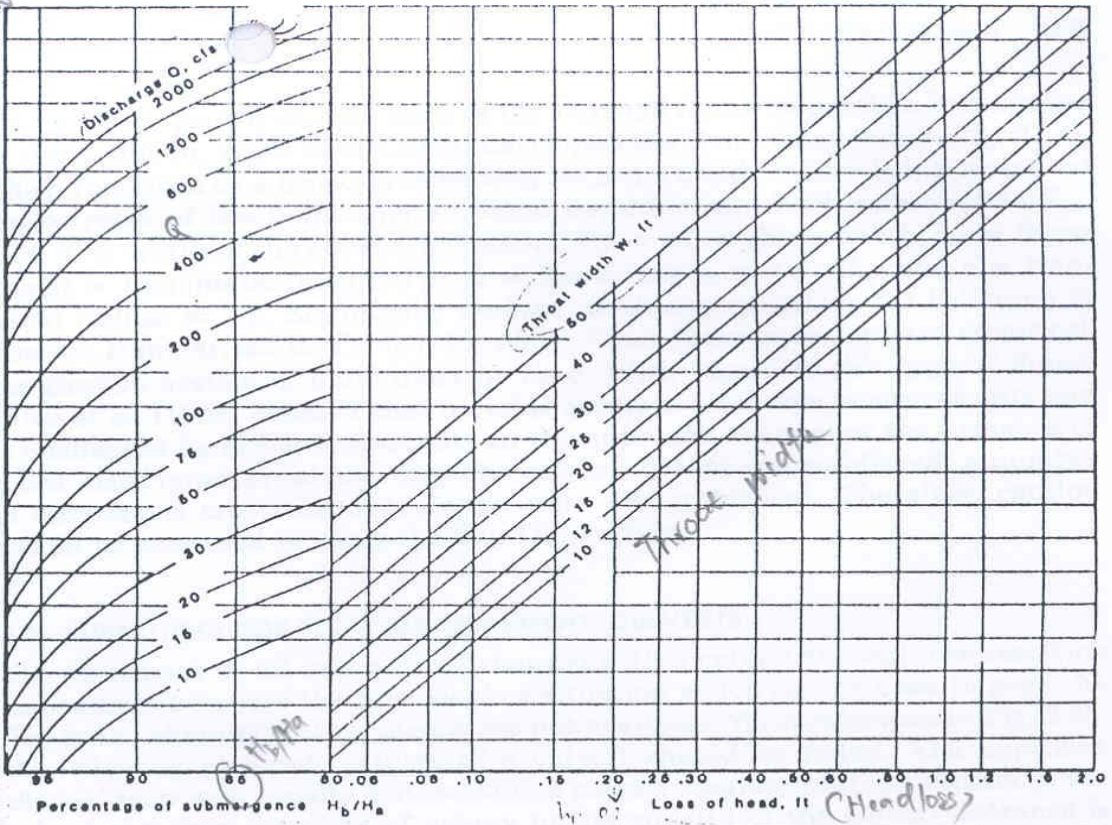
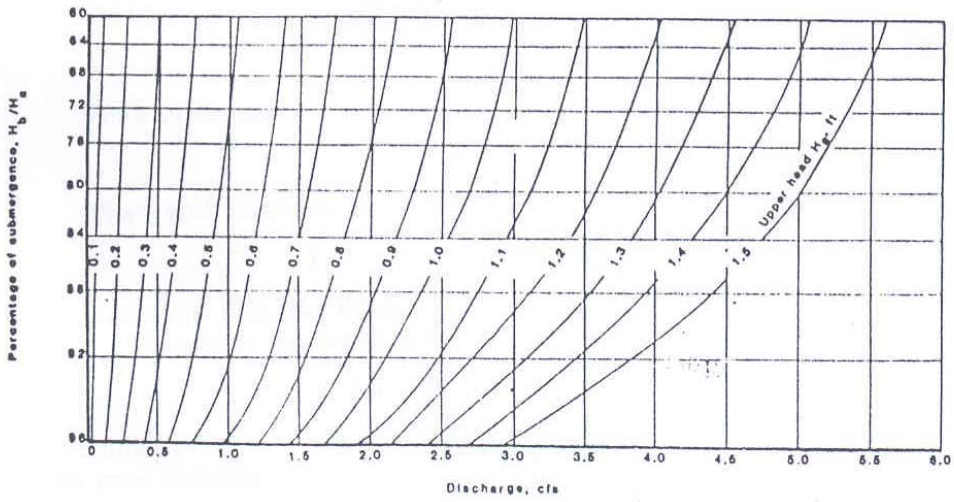
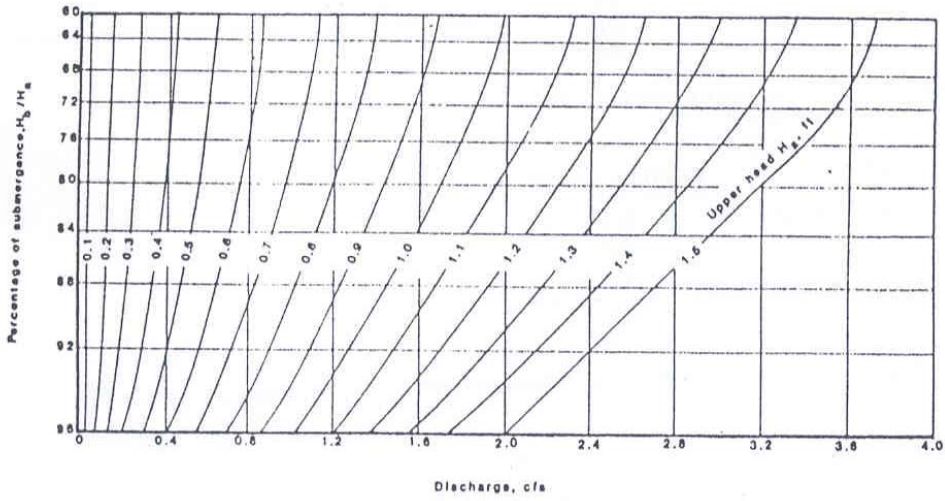
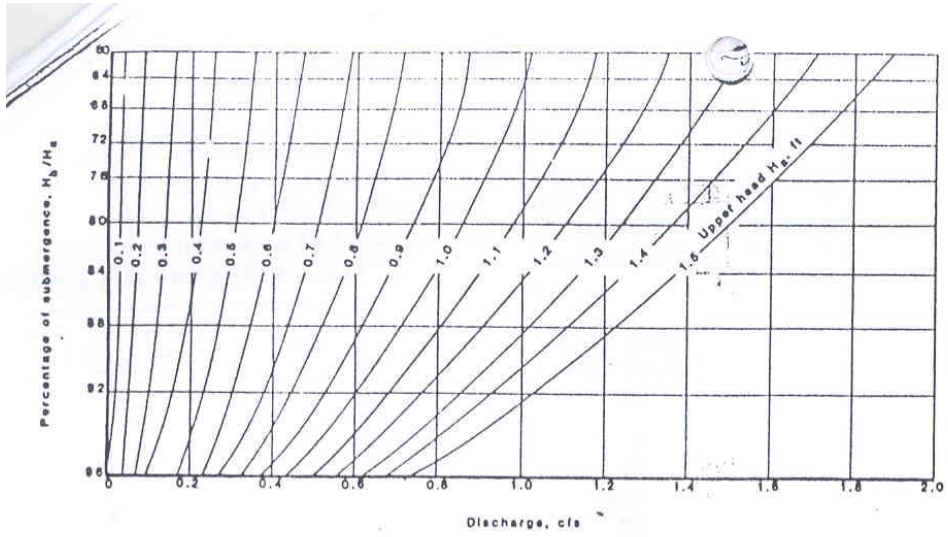
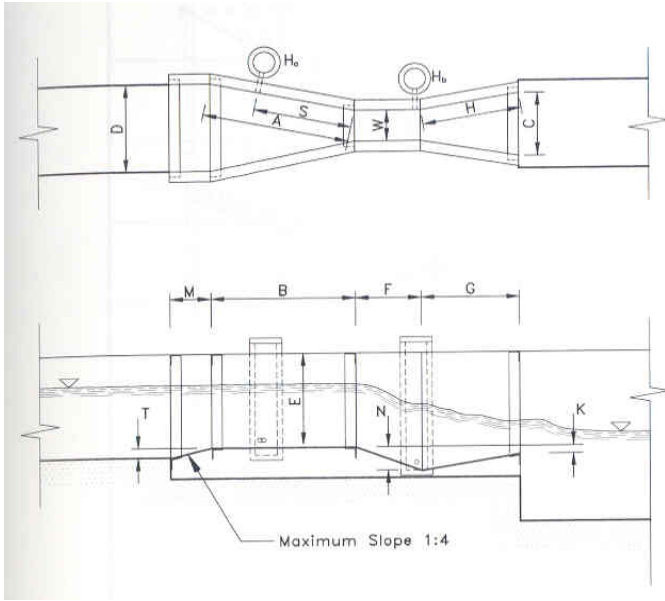


FIGURE 8.22 Diagrams for determining the loss of head through Parshall flumes of



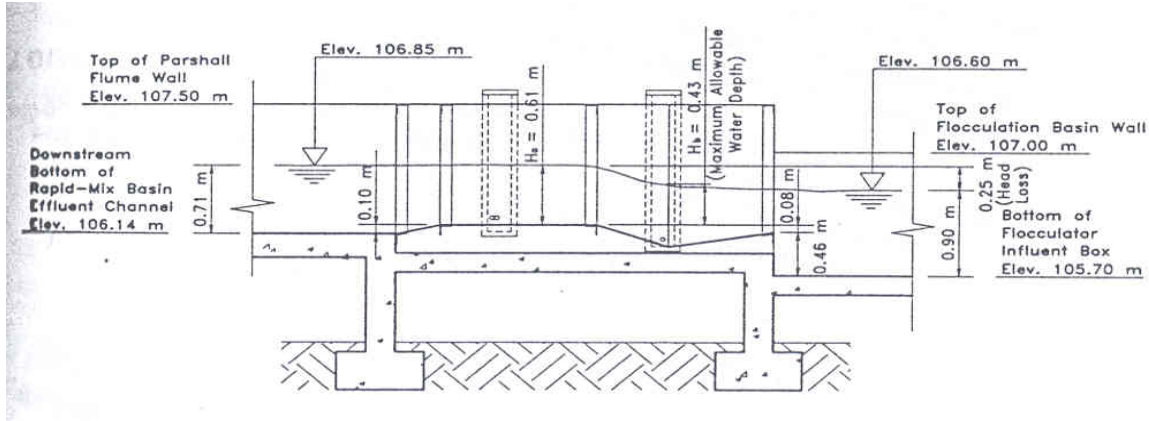


Standard Dimensions of Parshall Flume
 $W = 0.3 \text{ m (1 ft)}$

Section	Dimension, m
A	1.37
B	1.34
C	0.61
D	0.85
E	0.91
F	0.61
G	0.91
H	0.92
K	0.08
M	0.38
N	0.23
S	0.91
T	0.10
W	0.30

Flow Measurement

Maximum Capacity	456 L/s
Minimum Capacity	10 L/s



1 Calculate Parshall Flume

Give $\Delta H = 0.3$ m.

$Q = 4.86$ m³/s

$= 291.6$ m³/min

$V(\text{volume}) = 20$ m³

$\mu = 0.000798$ N.s/m² at 30 °C = Kg/m.s

$\rho = 995.7$ kg/m³ 30 °C

Throat width (W) = 10 ft

Theory $P = 0.1635Q\Delta H$ [KW]
 $= 14.30298$ KW
 $= 14302.98$ W = N.m/s

Theory $G = \sqrt{\frac{P(N.m/s)}{\mu(N.s/m^2)V(m^3)}}$
 $G = 947$ s⁻¹

2 Calculate H_b/H_a

Theory $G = \sqrt{\frac{P(N.m/s)}{\mu(N.s/m^2)V(m^3)}}$

$P = \rho g Q \Delta H$

Then $G = \sqrt{\frac{\rho(kg/m^3)g(m/s^2)Q(m^3/s)\Delta H(m)}{\mu(kg/m.s)V(m^3)}}$

$\therefore \Delta H = 0.301$ m.

$\Delta H = 0.989$ ft.

Then $\Delta H = 0.989$ ft at $Q = 4.86$ m³/s = 171.654 ft³/s

and Throat width= 10 ft Calculate H_b/H_a= 0.8 (From Graph 8.22)

1 Calculation Velocity Gradient

$$\begin{aligned} \text{Give } Q &= 4.86 \text{ m}^3/\text{s} \\ &= 171.65 \text{ ft}^3/\text{s} \\ \text{Throat width (W)} &= 10 \text{ ft} \end{aligned}$$

Theory 1

$$Q = 4WH_a^{1.522W^{0.026}} \quad (\text{Water Works Engineering, Qasim, 2000 Year})$$

Where :

$$\begin{aligned} Q &= \text{Free Flow discharge, ft}^3/\text{s} \\ W &= \text{Throat width, ft} \\ H_a &= \text{Depth of water at upstream gauging point, ft} \end{aligned}$$

$$\therefore H_a = 2.4631 \text{ ft} \quad \longrightarrow \quad (1)$$

Theory 2

$$Q = (3.6875W + 2.5)H_a^{1.6} \quad (\text{Open-Chanel Hydraulic Book})$$

This Equation use for Throat width $10 < W < 50$ ft and $\frac{H_b}{H_a} \geq 0.8$

Where :

$$\begin{aligned} Q &= \text{Free Flow discharge, ft}^3/\text{s} \\ W &= \text{Throat width, ft} \\ H_a &= \text{Depth of water at upstream gauging point, ft} \end{aligned}$$

$$\therefore H_a = 2.5098 \text{ ft} \quad \longrightarrow \quad (2)$$

$$(1) \approx (2) \quad \text{OK}$$

$$\therefore H_b = 1.9705 \text{ ft}$$

Theory

$$G = \sqrt{\frac{\rho(\text{kg}/\text{m}^3)g(\text{m}/\text{s}^2)Q(\text{m}^3/\text{s})\Delta H(\text{m})}{\mu(\text{kg}/\text{m}\cdot\text{s})V(\text{m}^3)}}$$

$$946.67 \text{ s}^{-1}$$